CONTRACT REPORT

arGb

ARRB Group Ltd www.arrb.com.au

ACN 004 620 651 ABN 68 004 620 651

- Research and Consulting
- Systems

FWD Testing Results for Intermodal Terminal

Project No: PSS16279

by Huimin Moore

for CPB Contractors Pty Ltd





FWD Testing Results for Intermodal Terminal

for CPB Contractors Pty Ltd

Reviewed Project Leader Clint Bradley Quality Manager Jim Johnson-Clarke

PSS16279 April 2017

ARRB Group Ltd ABN 68 004 620 651

....

500 Burwood Highway Vermont South VIC 3133 Australia P: +61 3 9881 1555 F: +61 3 9887 8104 info@arrb.com.au

Western Australia

191 Carr Place Leederville WA 6007 Australia P: +61 8 9227 3000 F: +61 8 9227 3030 arrb.wa@arrb.com.au

New South Wales

2-14 Mountain St Ultimo NSW 2007 Australia P: +61 2 9282 4444 F: +61 2 9280 4430 arrb.nsw@arrb.com.au

Queensland

123 Sandgate Road Albion QLD 4010 Australia P: +61 7 3260 3500 F: +61 7 3862 4699 arrb.qld@arrb.com.au

South Australia

Level 5, City Central, Suite 507, 147 Pirie Street Adelaide SA 5000 Australia P: +61 8 7200 2659 F: +61 8 8223 7406 arrb.sa@arrb.com.au

International office

770 Pennsylvania Drive Suite 112 Exton, PA 19341 USA Tel: 610-321-8302 Fax: 610-458-2467



FWD TESTING RESULTS FOR INTERMODAL TERMINAL



SUMMARY

ARRB Group Ltd (ARRB) was commissioned by CPB Contractors Pty Limited to collect the pre-construction FWD data of the proposed survey routes in relation to Intermodal Terminal.

The aim of the report is to present a basic summary of the FWD testing results as the pre-construction road condition of the study area.

The key findings of the baseline pre-construction condition assessment are as follows:

- Glenfield Road at 1.6km westbound lane 1 left wheel is found with over 1000 microns deflection, which might indicate potential weak pavement structure.
- All other sections are at good deflection state.



Although the Report is believed to be correct at the time of publication, ARRB Group Ltd, to the extent lawful, excludes all liability for loss (whether arising under contract, tort, statute or otherwise) arising from the contents of the Report or from its use. Where such liability cannot be excluded, it is reduced to the full extent lawful. Without limiting the foregoing, people should apply their own skill and judgement when using the information contained in the Report.

CONTENTS

1	INTRODUCTION	9
2	FWD TEST	10
3	DEFLECTION DATA PLOT	12
4	CONCLUSIONS	33
5	DATA FILES	34
RFF	FRENCES	35

TABLES

Table 1.1	Surveyed road sections	g
FIGURES		
Figure 1.1:	Survey routes	g
Figure 2.1:	Falling weight deflectometer (FWD)	
Figure 2.2:	Pavement deflection bowl (not to scale)	11
Figure 3.1:	Deflection (d0) for Cambridge Avenue Eastbound Lane 1 left wheel path	10
Figure 3.2:	Deflection (d0) for Cambridge Avenue Eastbound Lane 1 right wheel path	
Figure 3.3:	Deflection (d0) for Cambridge Avenue Eastbound Lane 2 left wheel path	
Figure 3.4:	Deflection (d0) for Cambridge Avenue Eastbound Lane 2 right wheel path	
Figure 3.5:	Deflection (d0) for Cambridge Avenue Westbound Lane 1 left wheel path	
Figure 3.6:	Deflection (d0) for Cambridge Avenue Westbound Lane 1 right wheel path	
Figure 3.7:	Deflection (d0) for Cambridge Avenue Westbound Lane 2 left wheel path	
Figure 3.8:	Deflection (d0) for Cambridge Avenue Westbound Lane 2 right wheel path	15
Figure 3.9:	Deflection (d0) for Campbelltown Road Northbound Lane 1 left wheel path	
Figure 3.10:	Deflection (d0) for Campbelltown Road Northbound Lane 1 right wheel path	
Figure 3.11:	Deflection (d0) for Campbelltown Road Northbound Lane 2 left wheel path	
Figure 3.12:	Deflection (d0) for Campbelltown Road Northbound Lane 2 right wheel path	
Figure 3.13:	Deflection (d0) for Campbelltown Road Northbound Lane 3 left wheel path	
Figure 3.14:	Deflection (d0) for Campbelltown Road Northbound Lane 3 right wheel path	
Figure 3.15:	Deflection (d0) for Campbelltown Road Southbound Lane 1 left wheel path	
Figure 3.16:	Deflection (d0) for Campbelltown Road Southbound Lane 1 right wheel path	
Figure 3.17:	Deflection (d0) for Campbelltown Road Southbound Lane 2 left wheel path	
Figure 3.18:	Deflection (d0) for Campbelltown Road Southbound Lane 2 right wheel path	
Figure 3.19:	Deflection (d0) for Campbelltown Road Southbound Lane 3 left wheel path	
Figure 3.20:	Deflection (d0) for Campbelltown Road Southbound Lane 3 right wheel path	
Figure 3.21:	Deflection (d0) for Campbelltown Road Southbound Lane 4 left wheel path	
Figure 3.22:	Deflection (d0) for Campbelltown Road Southbound Lane 4 right wheel	44
94.0 0.22.	path	22



Figure 3.23:	Deflection (d0) for Canterbury Road Roundabout Clockwise Lane 1 left wheel path	23
Figure 3.24:	Deflection (d0) for Canterbury Road Roundabout Clockwise Lane 1 right wheel path	23
Figure 3.25:	Deflection (d0) for Canterbury Road Roundabout Clockwise Lane 2 left	23
Figure 3.26:	wheel path Deflection (d0) for Canterbury Road Roundabout Clockwise Lane2 right wheel path	24
Figure 3.27:	Deflection (d0) for Glenfield Road Eastbound Lane 1 left wheel path	
Figure 3.28:	Deflection (d0) for Glenfield Road Eastbound Lane 1 right wheel path	
Figure 3.29:	Deflection (d0) for Glenfield Road Eastbound Lane 2 left wheel path	
Figure 3.30:	Deflection (d0) for Glenfield Road Eastbound Lane 2 right wheel path	
Figure 3.31:	` '	27
Figure 3.32:	Deflection (d0) for Glenfield Road Westbound Lane 1 right wheel path	27
Figure 3.33:	Deflection (d0) for Glenfield Road Westbound Lane 2 left wheel path	
Figure 3.34:	Deflection (d0) for Glenfield Road Westbound Lane 2 right wheel path	28
Figure 3.35:	Deflection (d0) for Glenfield Road Westbound Lane 3 left wheel path	29
Figure 3.36:	Deflection (d0) for Glenfield Road Westbound Lane 3 right wheel path	29
Figure 3.37:	Deflection (d0) for Moorebank Ave Northbound Lane 1 left wheel path	30
Figure 3.38:	Deflection (d0) for Moorebank Ave Northbound Lane 1 right wheel path	30
Figure 3.39:	Deflection (d0) for Moorebank Ave Southbound Lane 1 left wheel path	31
Figure 3.40:	Deflection (d0) for Moorebank Ave Southbound Lane 1 right wheel	
	path	31
Figure 3.41:	Deflection (d0) for Railway Station Roundabout Clockwise Lane 1 left wheel path	32
Figure 3.42:	Deflection (d0) for Railway Station Roundabout Clockwise Lane 1 right	02
94.0 02.	wheel nath	32

1 INTRODUCTION

ARRB Group Ltd (ARRB) was commissioned by CPB Contractors Pty Limited to collect the preconstruction FWD data of the proposed survey routes in relation to Intermodal Terminal.

The aim of the report is to present a basic summary of the FWD testing results as the preconstruction road condition of the study area.

The testing included all wheel paths and lanes of the selected road sections as listed in Table 1.1.

ROAD	SECTION
Cambridge Ave	Moorebank Ave - Railway Overpass
Campbelltown Rd	Glenfield Rd - Camden Valley Way
Canterbury Rd	
Roundabout	Canterbury Rd CL - Canterbury Rd CL
	Railway Overpass - Railway Station
Glenfield Rd	Roundabout
Moorebank Ave	Railway Overpass - Cambridge Ave
Railway Station	Glenfield Rd (from Cambridge) -
Roundabout	Glenfield Rd (to Cambridge)

Table 1.1 Surveyed road sections

Figure 1.1 presents the surveyed routes on the map.

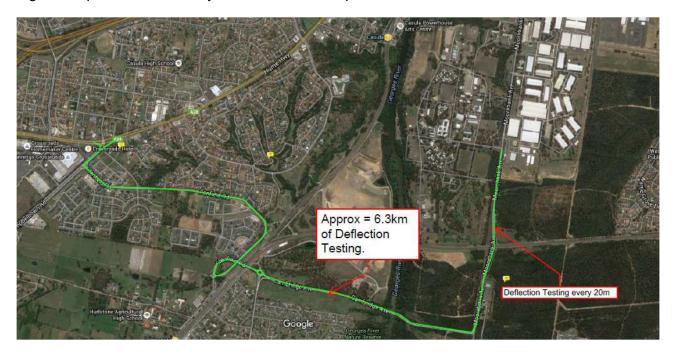


Figure 1.1: Survey routes

TC-423-1-3-2 - 9 - April 2017

2 FWD TEST

The FWD testing is a form of non-destructive testing used to evaluate the physical properties of the pavement. Data is collected by the measurement of a dynamic load that is generated by dropping a mass from a pre-set height onto a 300 mm diameter plate (Figure 2.1). The machine is stopped at every test point (100m intervals for current work) with an impulse load delivered to the pavement by the applied load equivalent to the weight of a 10-tonne axle momentarily loading the road (as would happen for a large truck passing at about 80 km/h).



Figure 2.1: Falling weight deflectometer (FWD)

Source: ARRB Group Ltd

When the applied load hits the pavement surface, the surface deflects to form what is called a 'deflection bowl' (Figure 2.2). This bowl is automatically measured by the FWD sensors. The bowl shape is analysed to determine the condition of the pavement from a structural point of view. The numbers involved are very small, with a deflection of only about 1 mm, but when these deflections and the consequent bowl shape are measured very accurately they provide a lot of useful information about the nature and condition of the pavement.

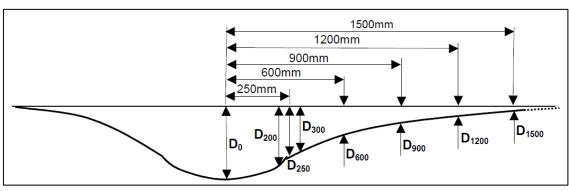


Figure 2.2: Pavement deflection bowl (not to scale)

Source: Austroads (2005)

Various deflection data (e.g., d0, d200 and d900 in microns) from a particular point provide a basis for assessment of the pavement strength of that location; d200 (and d0) together are used to assess the relative contribution to the overall strength by the upper portions of the pavement (the pavement stiffness). The value of d900 provides information about the lower pavement strength, in particular the founding natural subgrade.

The concrete road sections have been excluded from FWD testing. For the selected road sections, variation of d0 over the length of the sections have been graphically presented in the following figures. These figures give an overview of overall pavement deflection showing variations over the length of the sections.

General knowledge suggests testing points with over 1000 microns d0 values might indicate relatively weak pavement section. This could potentially be caused due to localised weak sections, cracking, under-road pipes etc.

3 DEFLECTION DATA PLOT

Figure 3.1: Deflection (d0) for Cambridge Avenue Eastbound Lane 1 left wheel path

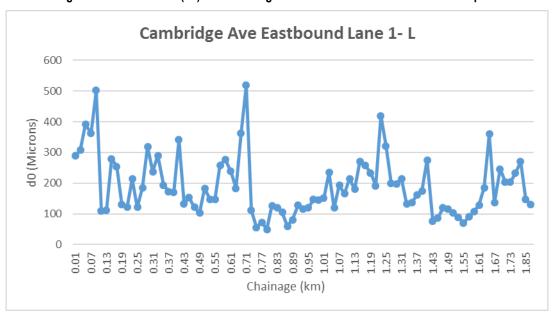
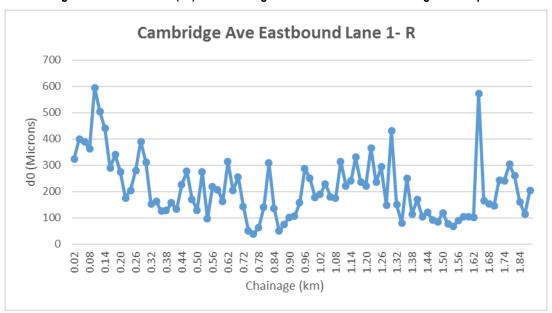


Figure 3.2: Deflection (d0) for Cambridge Avenue Eastbound Lane 1 right wheel path



TC-423-1-3-2 - 12 - April 2017

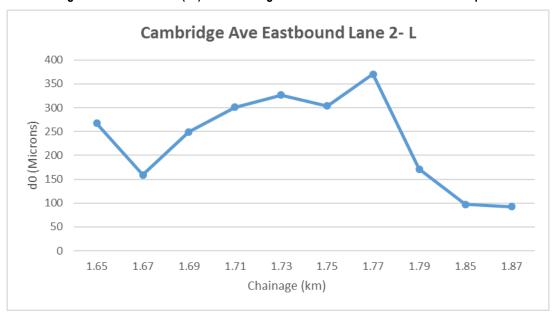
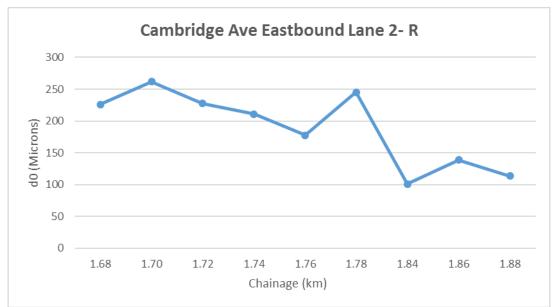


Figure 3.3: Deflection (d0) for Cambridge Avenue Eastbound Lane 2 left wheel path





TC-423-1-3-2 - 13 - April 2017

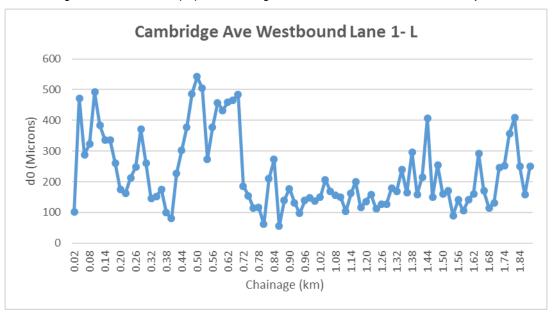
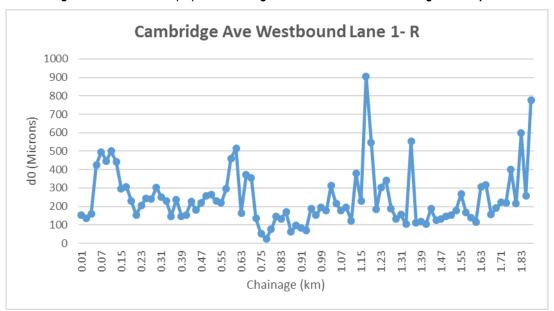


Figure 3.5: Deflection (d0) for Cambridge Avenue Westbound Lane 1 left wheel path





TC-423-1-3-2 - 14 - April 2017

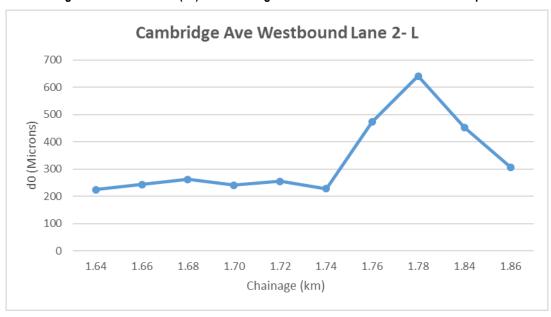
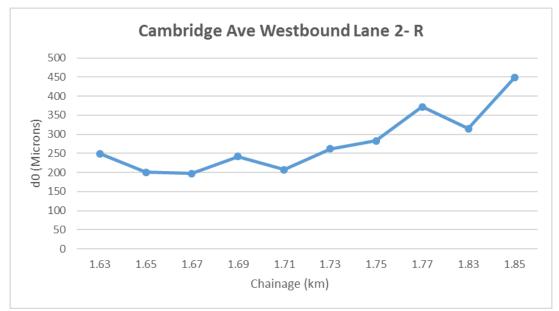


Figure 3.7: Deflection (d0) for Cambridge Avenue Westbound Lane 2 left wheel path





TC-423-1-3-2 - 15 - April 2017

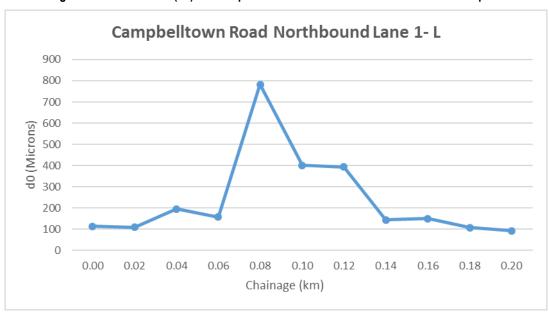
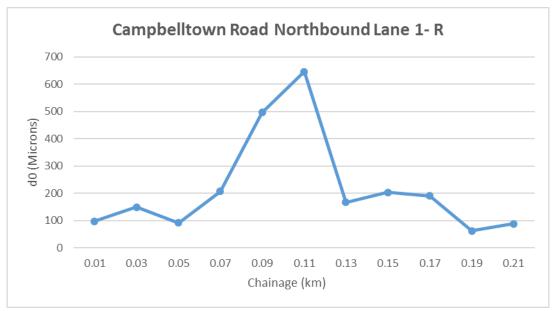


Figure 3.9: Deflection (d0) for Campbelltown Road Northbound Lane 1 left wheel path





TC-423-1-3-2 - 16 - April 2017

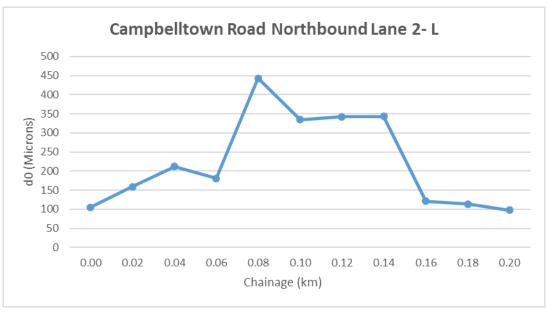
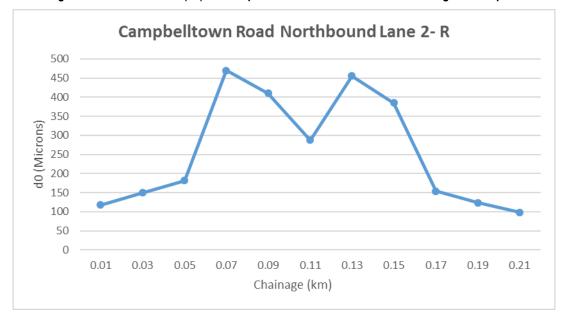


Figure 3.11: Deflection (d0) for Campbelltown Road Northbound Lane 2 left wheel path





TC-423-1-3-2 - 17 - April 2017

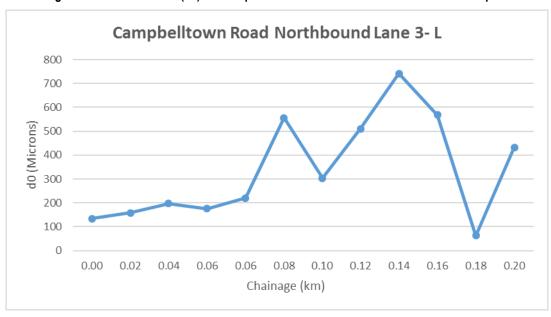
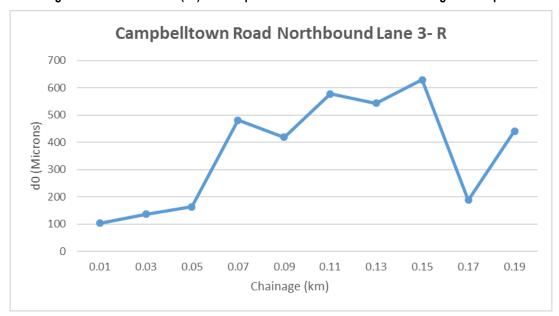


Figure 3.13: Deflection (d0) for Campbelltown Road Northbound Lane 3 left wheel path





TC-423-1-3-2 - 18 - April 2017

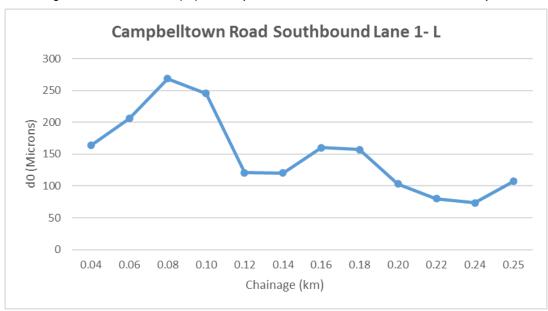
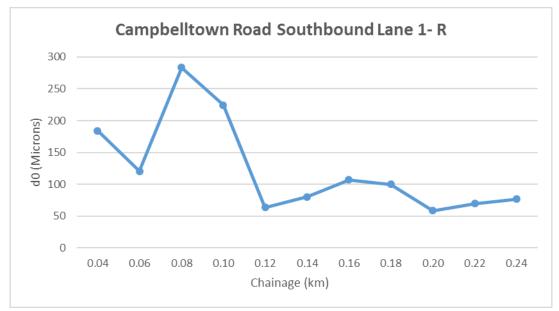


Figure 3.15: Deflection (d0) for Campbelltown Road Southbound Lane 1 left wheel path





TC-423-1-3-2 - 19 - April 2017

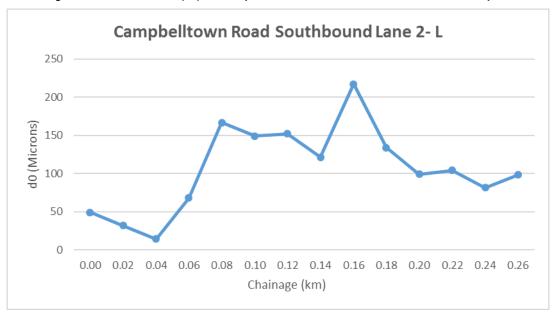
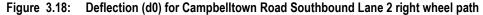
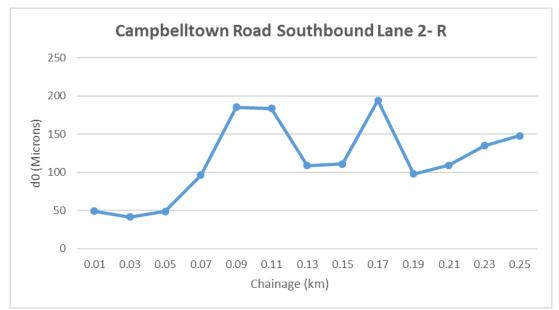


Figure 3.17: Deflection (d0) for Campbelltown Road Southbound Lane 2 left wheel path



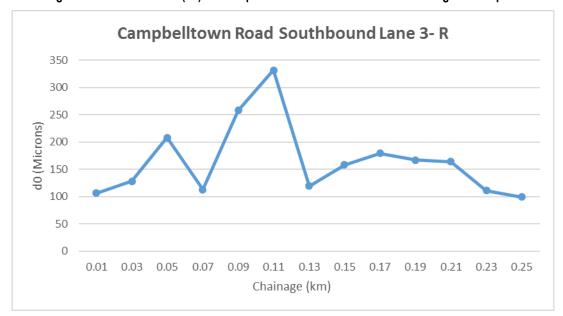


TC-423-1-3-2 - 20 - April 2017

Campbelltown Road Southbound Lane 3-L 350 300 250 do (Microns) 200 100 50 0 0.00 0.02 0.04 0.06 0.08 0.10 0.12 0.14 0.16 0.18 0.20 0.22 0.24 Chainage (km)

Figure 3.19: Deflection (d0) for Campbelltown Road Southbound Lane 3 left wheel path





TC-423-1-3-2 - 21 - April 2017

Campbelltown Road Southbound Lane 4- L

700
600
500
200
100
0.20
Chainage (km)

Figure 3.21: Deflection (d0) for Campbelltown Road Southbound Lane 4 left wheel path





TC-423-1-3-2 - 22 - April 2017

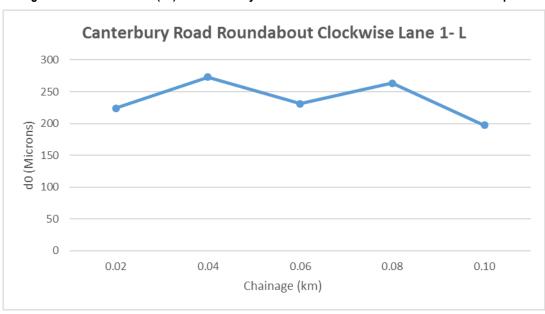
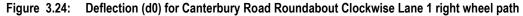
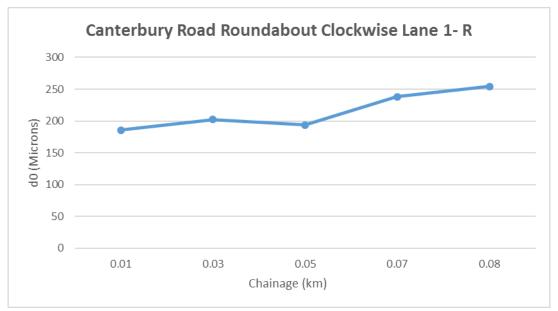


Figure 3.23: Deflection (d0) for Canterbury Road Roundabout Clockwise Lane 1 left wheel path





TC-423-1-3-2 - 23 - April 2017

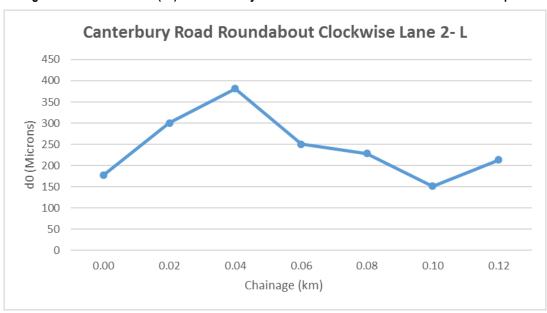
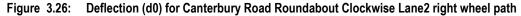
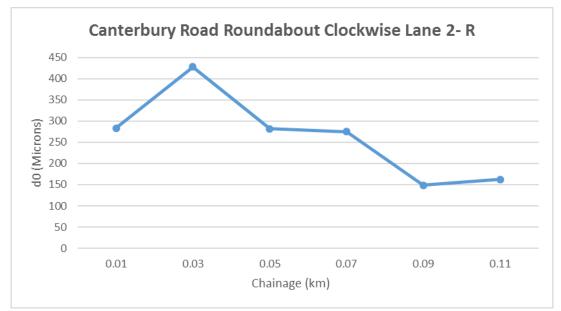


Figure 3.25: Deflection (d0) for Canterbury Road Roundabout Clockwise Lane 2 left wheel path





TC-423-1-3-2 - 24 - April 2017

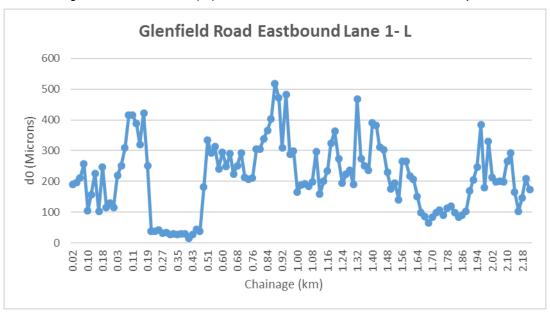
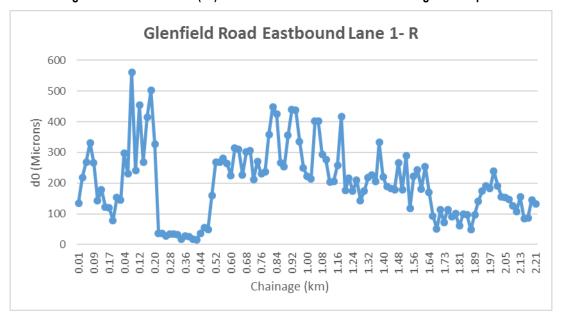


Figure 3.27: Deflection (d0) for Glenfield Road Eastbound Lane 1 left wheel path





TC-423-1-3-2 - 25 - April 2017

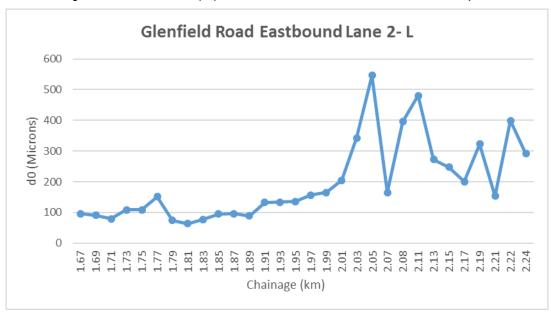
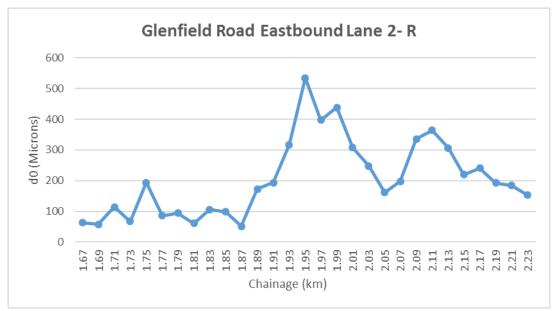


Figure 3.29: Deflection (d0) for Glenfield Road Eastbound Lane 2 left wheel path





TC-423-1-3-2 - 26 - April 2017

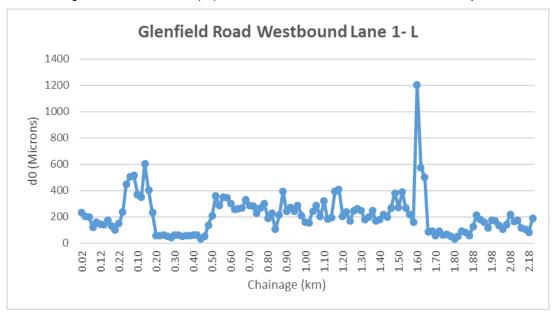
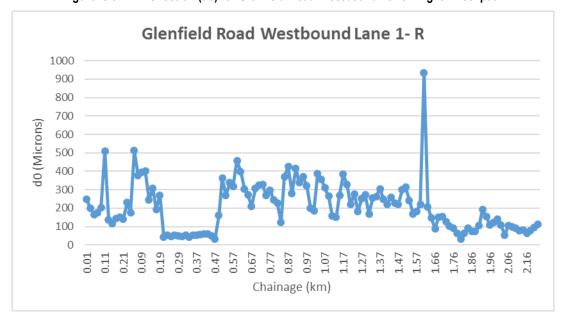


Figure 3.31: Deflection (d0) for Glenfield Road Westbound Lane 1 left wheel path





TC-423-1-3-2 - 27 - April 2017

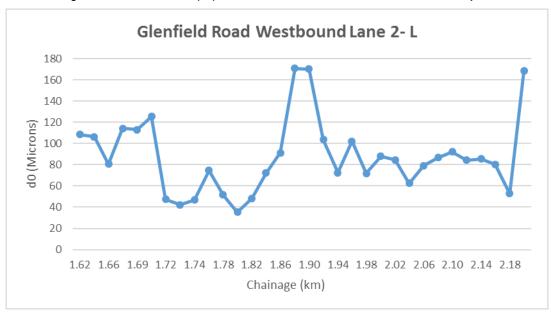
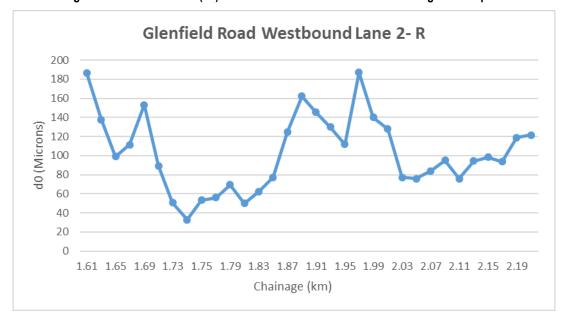


Figure 3.33: Deflection (d0) for Glenfield Road Westbound Lane 2 left wheel path





TC-423-1-3-2 - 28 - April 2017

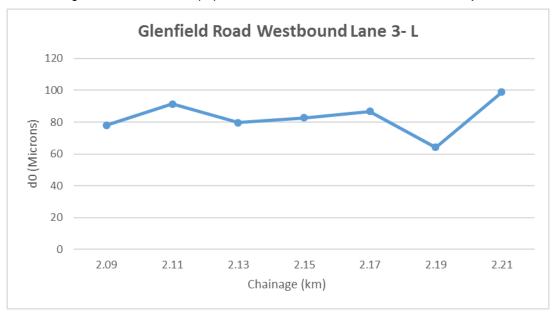
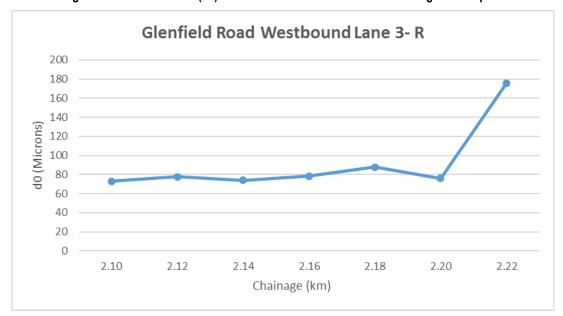


Figure 3.35: Deflection (d0) for Glenfield Road Westbound Lane 3 left wheel path





TC-423-1-3-2 - 29 - April 2017

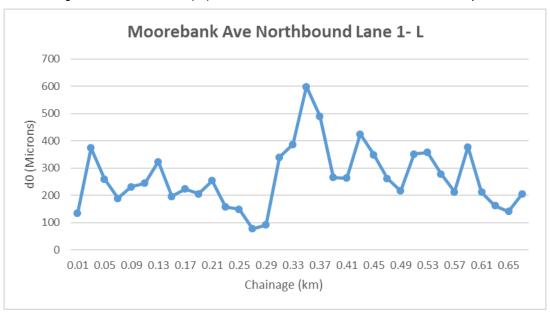
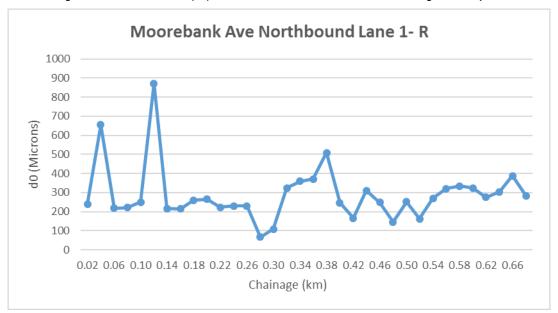


Figure 3.37: Deflection (d0) for Moorebank Ave Northbound Lane 1 left wheel path

Figure 3.38: Deflection (d0) for Moorebank Ave Northbound Lane 1 right wheel path



TC-423-1-3-2 - 30 - April 2017

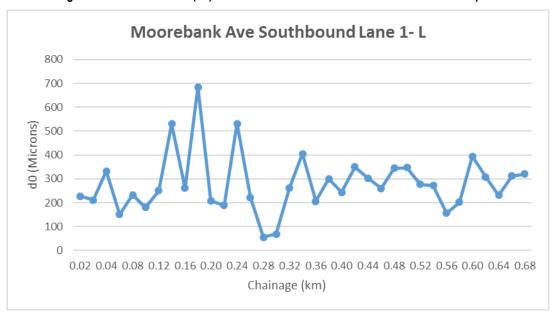
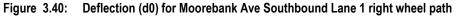
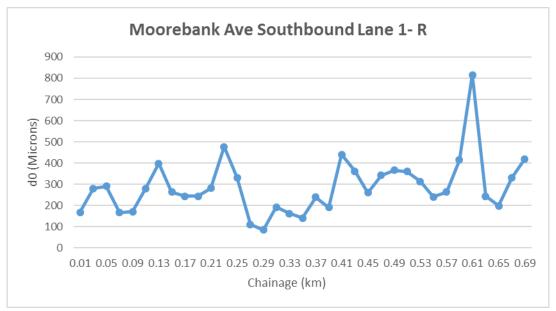


Figure 3.39: Deflection (d0) for Moorebank Ave Southbound Lane 1 left wheel path





TC-423-1-3-2 - 31 - April 2017

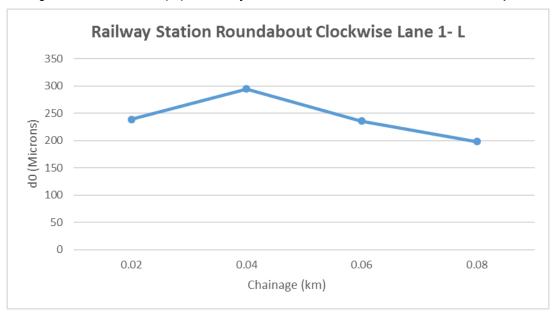
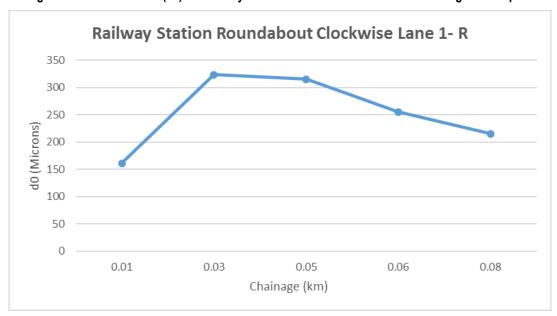


Figure 3.41: Deflection (d0) for Railway Station Roundabout Clockwise Lane 1 left wheel path





Glenfield Road westbound lane 1 left wheel at 1.6km is found with over 1000 microns deflection, which might indicate potential weak pavement structure. Road sections with close to 1000 microns deflection points include 1.6km of Glenfield Road westbound lane 1 right wheel path and Cambridge Avenue westbound lane 1 right lane.

TC-423-1-3-2 - 32 - April 2017

4 CONCLUSIONS

The findings of the new baseline pre-construction condition assessment are as follows:

- Glenfield Road at 1.6km westbound lane 1 left wheel is found with over 1000 microns deflection, which might indicate potential weak pavement structure.
- All other sections are at good deflection state.

TC-423-1-3-2 - 33 - April 2017

5 DATA FILES

The following data files have been delivered:

Survey date	Data files
29/03/2017	PSS16279 FWD_CPB Contractors_Intermodal Terminal.xlsx